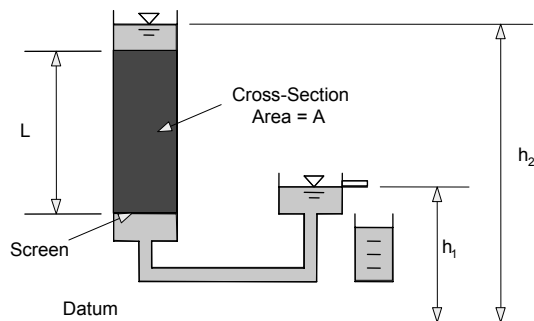


Darcy's Law

Brigham Young University
CE En 547

Darcy's Experiment

- Darcy did some lab tests on flow through sand in 1856



Darcy's Law

Darcy experimented with different soils and with different values of L , h_1 , and h_2 . The results showed that:

$$q = kA \frac{h_2 - h_1}{L}$$

where:

$q = \Delta \text{Vol} / \Delta t = \text{flow rate [L}^3/\text{T]}$

$k = \text{coefficient of permeability}$

or hydraulic conductivity [L/T]

$A = \text{gross cross-sectional area of flow [L}^2\text{]}$

$h = \text{total head [L]}$

$L = \text{length of flow path [L]}$

Alternate Formulation

Darcy's law is often written as:

$$q = kiA$$

where

$i = \text{hydraulic gradient}$

$$i = \frac{h_2 - h_1}{L} = \frac{\Delta h}{L}$$

$$i = -\frac{dh}{ds} \quad \text{where } s \text{ is the flow path length}$$

$$q = -k \frac{dh}{ds} A$$

Similar Governing Equations

- There are other governing equations that have this same form.
- Examples
 - Flow of electricity (Ohm's Law)
 - Heat flow

Ohm's Law

George Ohm (1789-1854) did a series of tests like Darcy did and came up with "Ohm's Law"

$$i = -K \frac{V_a - V_b}{L} A = -K \frac{dV}{dx} A$$

where

i = current
K = electrical conductivity
K = 1/ρ where ρ = resistivity
V = voltage
L = distance
A = area

Sometimes written as:

V = IR
where
R = ρ(L/A)

Heat Flow

$$q = -K \frac{dT}{dx} A$$

where

q = rate of heat flow

K = thermal conductivity

T = temperature

x = distance

A = cross-sectional area

Applicability

- Darcy's law is only valid for laminar flow.
- Upper bound for laminar flow is based on Reynold's number
 - $Re > \sim 1$

Reynold's Number

$$R_e = \frac{vD\rho}{\mu}$$

where

v = discharge velocity [ft/s]

D = mean particle size [ft]

ρ = mass density of water [1.94 lb-sec²/ft⁴]

μ = dynamic viscosity of water [2.35X10⁻⁵ lb-sec/ft²]

Velocity

Darcy's law can be rewritten as:

$$q = kiA$$

$$\frac{q}{A} = ki \frac{A}{A}$$

$$v_d = ki$$

Darcian Velocity

v_d = "discharge velocity" or "Darcian velocity"



v_s = "seepage velocity"

Seepage Velocity

Seepage velocity can be related to discharge velocity as follows:

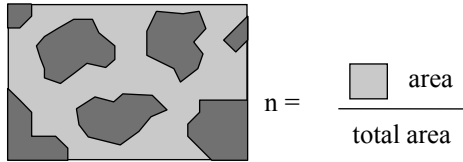
$$\frac{\text{Area of flow}}{\text{Total area}} = \frac{A_f}{A} = \frac{V_v}{V} = n$$

V_v = Volume of soil voids

V = Total volume

n = porosity

Seepage Velocity, cont.



$$v_s = \frac{q}{A_f} = \frac{q A}{A A_f} = v_d \frac{1}{n}$$

$$v_s = \frac{v_d}{n} = \frac{ki}{n}$$

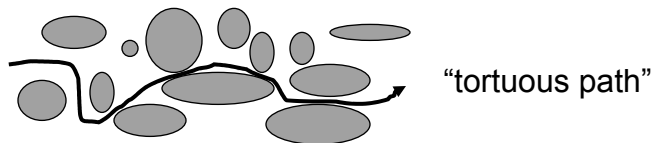
n	v_s/v_d
0.33	3
0.5	2
0.67	1.5
0.75	1.3

Tortuosity

Note: v_s is average linear velocity



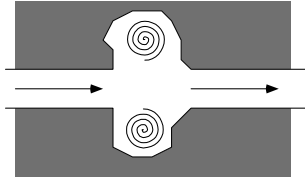
Actual velocity is higher because the water follows a longer path through the soil particles.



“Tortuosity” is a measure of the distance traveled vs. linear distance.

Effective Porosity

Not all voids in the soil conduct flow.



n_e = "effective" porosity

$$= \frac{A_f}{A}$$

where A_f = actual area of flow

$$v_s = \frac{ki}{n_e}$$

Effective Porosity, cont.

$$n_e = \lambda n$$

λ = effective porosity factor

λ is determined experimentally

$\lambda \approx 1$ for sands and gravels

$\lambda = 0.01 - 0.5$ for clays

Permeability

In the equation for Darcy's law:

$$q = kiA$$

k is a constant which depends on both the fluid and the soil.

Darcy's law can be rewritten as:

$$q = -K \frac{\gamma_f}{\mu} \frac{dh}{dx} A$$

where:

K = "constant of proportionality" [L²] or "intrinsic permeability" or "permeability"

K is based on soil properties only

γ_f = unit weight of fluid [F/L³]

μ = viscosity of fluid [ML/T]

Permeability, cont.

Can also be written as:

$$q = \frac{K}{\mu} \left[\rho g \frac{dh}{dx} \right] A$$

This form is used a lot in the petroleum industry.

K is often expressed in units of Darcys

1 Darcy = the permeability for a flow of 1 cm³/sec/cm² for a viscosity of 1 centipoise and $[\rho g(dh/dx)] = 1 \text{ atm/cm}$.

1 poise = 1 dyne-sec/cm²
1 Darcy = 0.987X10⁻⁸ cm²

For water at 20° C:

$$K = \frac{k\mu_w}{\gamma_w}$$

$\mu_w = 0.01002 \text{ poise}$

$\gamma_w = \rho_w g$

$\rho_w = 1 \text{ g/cm}^3$

$g = 980.7 \text{ cm/sec}^2$

for $k = 1 \text{ cm/s}$: $K = 1.02 \times 10^{-5} \text{ cm}^2$

See handout for tables of typical values of k & K
We will use k (hydraulic conductivity) in this class.

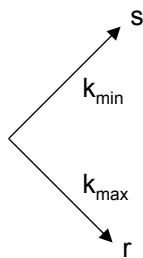
Darcy's Law in 2D

- Isotropic medium

$$v_x = -k \frac{\partial h}{\partial x}$$

$$v_y = -k \frac{\partial h}{\partial y}$$

Anisotropic Medium



$$v_r = -k_{\max} \frac{\partial h}{\partial r}$$

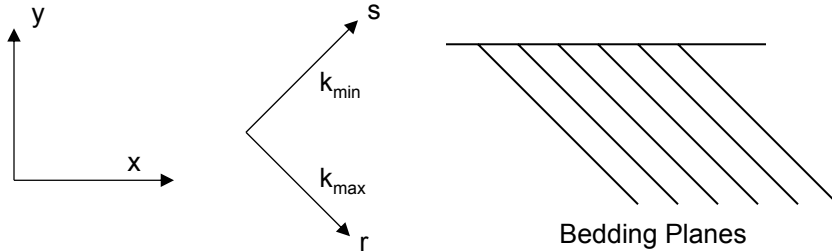
$$v_s = -k_{\min} \frac{\partial h}{\partial s}$$

$$\begin{bmatrix} v_r \\ v_s \end{bmatrix} = - \begin{bmatrix} k_{\max} & 0 \\ 0 & k_{\min} \end{bmatrix} \begin{bmatrix} \partial h / \partial r \\ \partial h / \partial s \end{bmatrix}$$

$$\begin{bmatrix} k_{\max} & 0 \\ 0 & k_{\min} \end{bmatrix} = \text{"conductivity matrix" or "conductivity tensor"}$$

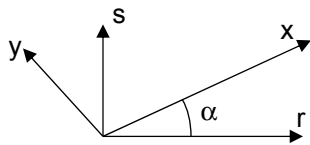
r & s may or may not coincide with x & y

Example



You often need k in terms of x & y (k_x, k_y).

Coordinate Transformation



$$v_x = -k_{xx} \frac{\partial h}{\partial x} - k_{xy} \frac{\partial h}{\partial y}$$

$$\begin{bmatrix} v_x \\ v_y \end{bmatrix} = - \begin{bmatrix} k_{xx} & k_{xy} \\ k_{yx} & k_{yy} \end{bmatrix} \begin{bmatrix} \partial h / \partial x \\ \partial h / \partial y \end{bmatrix}$$

$$v_y = -k_{yx} \frac{\partial h}{\partial x} - k_{yy} \frac{\partial h}{\partial y}$$

Coord. Transformation, cont.

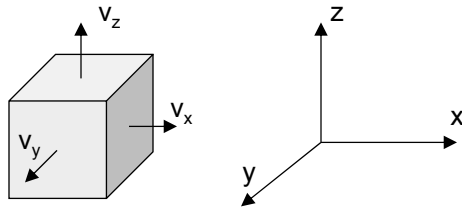
Solving in terms of k_r and k_s :

$$k_{xx} = k_r \cos^2 \alpha + k_s \sin^2 \alpha$$

$$k_{yy} = k_r \sin^2 \alpha + k_s \cos^2 \alpha$$

$$k_{xy} = k_{yx} = -\frac{1}{2}(k_r - k_s) \sin^2 \alpha$$

Darcy's Law in 3D



If x , y , and z axes coincide with principal axes of permeability:

$$\begin{bmatrix} v_x \\ v_y \\ v_z \end{bmatrix} = \begin{bmatrix} k_{xx} & 0 & 0 \\ 0 & k_{yy} & 0 \\ 0 & 0 & k_{zz} \end{bmatrix} \begin{bmatrix} \partial h / \partial x \\ \partial h / \partial y \\ \partial h / \partial z \end{bmatrix}$$

In most cases

$$\begin{aligned} k_{xx} &= k_{yy} \\ k_{xx} &> k_{zz} \end{aligned}$$

Darcy's Law in 3D, cont.

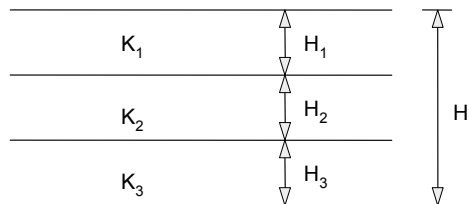
General case:

$$\begin{bmatrix} v_x \\ v_y \\ v_z \end{bmatrix} = \begin{bmatrix} k_{xx} & k_{xy} & k_{xz} \\ k_{yx} & k_{yy} & k_{yz} \\ k_{zx} & k_{zy} & k_{zz} \end{bmatrix} \begin{bmatrix} \partial h / \partial x \\ \partial h / \partial y \\ \partial h / \partial z \end{bmatrix}$$

The tensor is symmetric, i.e.:

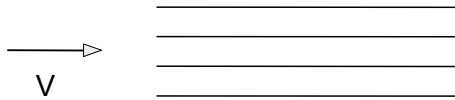
$$\begin{aligned} k_{yx} &= k_{xy} \\ k_{zx} &= k_{xz} \\ k_{zy} &= k_{yz} \end{aligned}$$

Layered Systems



It is useful to compute an equivalent k for entire system

Flow Parallel to Layering



For this case, the following must hold:

1. Head loss is the same through each layer

$$i_1 = i_2 = i_3$$

2. $q_{\text{total}} = q_1 + q_2 + q_3$

Parallel Flow, cont.

Thus,

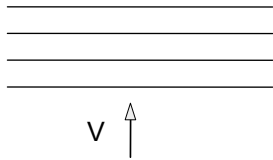
$$\begin{aligned} q &= q_1 + q_2 + q_3 \\ vH &= v_1H_1 + v_2H_2 + v_3H_3 \\ k_{\text{eq}}i_{\text{eq}}H &= k_1i_1H_1 + k_2i_2H_2 + k_3i_3H_3 \end{aligned} \quad (\text{width} = 1)$$

since i 's are the same, divide by i :

$$k_{\text{eq}}H = k_1H_1 + k_2H_2 + k_3H_3$$

$$k_{\text{eq}} = \frac{k_1H_1 + k_2H_2 + k_3H_3}{H} \quad k_{\text{eq}} = \frac{\sum_{i=1}^n k_i H_i}{\sum_{i=1}^n H_i}$$

Flow Perpendicular to Layering



For this case, the following must hold:

1. q will be the same through each layer
2. $\Delta h_{\text{total}} = \Delta h_1 + \Delta h_2 + \Delta h_3$

Perpendicular Flow, cont.

$$q = q_1 = q_2 = q_3$$

$$v = v_1 = v_2 = v_3$$

$$k_{\text{eq}} i_{\text{eq}} = k_1 i_1 = k_2 i_2 = k_3 i_3$$

$$k_{\text{eq}} (\Delta h / H) = k_1 i_1 = k_2 i_2 = k_3 i_3$$

$$\Delta h = \Delta h_1 + \Delta h_2 + \Delta h_3$$

$$\Delta h = H_1 i_1 + H_2 i_2 + H_3 i_3$$

Perpendicular Flow, cont.

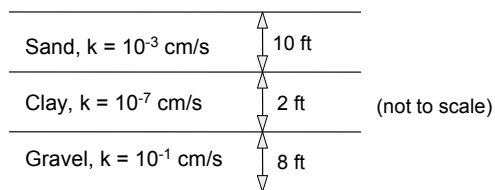
$$i_1 = \frac{k_{eq}\Delta h}{Hk_1} \quad i_2 = \frac{k_{eq}\Delta h}{Hk_2} \quad i_3 = \frac{k_{eq}\Delta h}{Hk_3}$$

$$\Delta h = \frac{H_1 k_{eq} \Delta h}{Hk_1} + \frac{H_2 k_{eq} \Delta h}{Hk_2} + \frac{H_3 k_{eq} \Delta h}{Hk_3}$$

Divide by Δh and solve for k_{eq} :

$$k_{eq} = \frac{H}{\frac{H_1}{k_1} + \frac{H_2}{k_2} + \frac{H_3}{k_3}} \quad k_{eq} = \frac{\sum_{i=1}^n H_i}{\sum_{i=1}^n \frac{H_i}{k_i}}$$

Example



k_{eq} (horizontal) = ?

k_{eq} (vertical) = ?